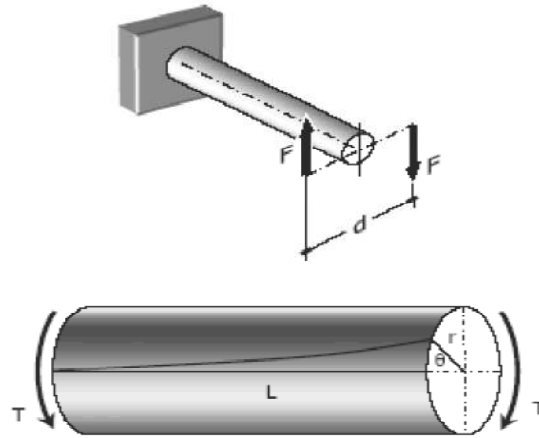




Strength of Material-5

Torsion and SF & BM

Dr. Attaullah Shah



Consider a bar to be rigidly attached at one end and twisted at the other end by a torque or twisting moment T equivalent to $F \times d$, which is applied perpendicular to the axis of the bar, as shown in the figure. Such a bar is said to be in torsion.

TORSIONAL SHEARING STRESS,

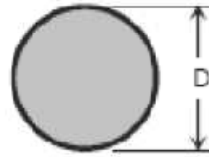
For a solid or hollow circular shaft subject to a twisting moment T , the torsional shearing stress τ at a distance ρ from the center of the shaft is

$$\tau = \frac{T\rho}{J} \text{ and } \tau_{\max} = \frac{Tr}{J}$$

where J is the polar moment of inertia of the section and r is the outer radius.

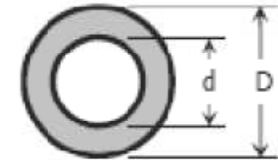
For solid cylindrical shaft:

$$J = \frac{\pi D^4}{32}$$
$$\tau_{\max} = \frac{16T}{\pi D^3}$$



For hollow cylindrical shaft:

$$J = \frac{\pi}{32}(D^4 - d^4)$$
$$\tau_{\max} = \frac{16TD}{\pi(D^4 - d^4)}$$



ANGLE OF TWIST

The angle θ through which the bar length L will twist is where T is the torque in N·mm, L is the length of shaft modulus in MPa, J is the polar moment of inertia in mm^4 , D and d are diameter in mm, and r is the radius in mm.

$$\theta = \frac{TL}{JG} \text{ in radians}$$

POWER TRANSMITTED BY THE SHAFT

A shaft rotating with a constant angular velocity ω (in radians per second) is being acted by a twisting moment T . The power transmitted by the shaft is

$$P = T\omega = 2\pi T f$$

where T is the torque in N·m, f is the number of revolutions per second, and P is the power in watts.

Solved Problems

- 304 A steel shaft 3 ft long that has a diameter of 4 in. is subjected to a torque of 15 kip·ft. Determine the maximum shearing stress and the angle of twist. Use $G = 12 \times 10^6$ psi.

$$\tau_{\max} = \frac{16T}{\pi D^3} = \frac{16(15)(1000)(12)}{\pi(4^3)}$$

$$\tau_{\max} = 14\,324 \text{ psi}$$

$$\tau_{\max} = \mathbf{14.3 \text{ ksi}}$$

$$\theta = \frac{TL}{JG} = \frac{15(3)(1000)(12^2)}{\frac{1}{32}\pi(4^4)(12 \times 10^6)}$$

$$\theta = 0.0215 \text{ rad}$$

$$\theta = \mathbf{1.23^\circ}$$

- **Problem 305** What is the minimum diameter of a solid steel shaft that will not twist through more than 3° in a 6-m length when subjected to a torque of 12 kN·m? What maximum shearing stress is developed? Use $G = 83 \text{ GPa}$.

$$\theta = \frac{TL}{JG}$$

$$3^\circ \left(\frac{\pi}{180^\circ} \right) = \frac{12(6)(1000^3)}{\frac{1}{32} \pi d^4 (83000)}$$

$$d = 113.98 \text{ mm}$$

$$\tau_{\max} = \frac{16T}{\pi d^3} = \frac{16(12)(1000^2)}{\pi(113.98^3)}$$

$$\tau_{\max} = 41.27 \text{ MPa}$$

- Problem 306** A steel marine propeller shaft 14 in. in diameter and 18 ft long is used to transmit 5000 hp at 189 rpm. If $G = 12 \times 10^6$ psi, determine the maximum shearing stress.

$$\theta = \frac{TL}{JG}$$

$$4^\circ \left(\frac{\pi}{180^\circ} \right) = \frac{T(5)(1000)}{\frac{1}{32} \pi d^4 (83000)}$$

$$T = 0.1138d^4$$

$$\tau_{\max} = \frac{16T}{\pi d^3}$$

$$80 = \frac{16(0.1138d^4)}{\pi d^3}$$

$$d = 138 \text{ mm}$$

$$T = \frac{P}{2\pi f}$$

$$0.1138d^4 = \frac{P}{2\pi(20)}$$

$$P = 14.3d^4 = 14.3(138^4)$$

$$P = 5\,186\,237\,285 \text{ N}\cdot\text{mm}/\text{sec}$$

$$P = 5\,186\,237.28 \text{ W}$$

$$P = 5.19 \text{ MW}$$

- **Problem 308** A 2-in-diameter steel shaft rotates at 240 rpm. If the shearing stress is limited to 12 ksi, determine the maximum horsepower that can be transmitted.

- **Solution 308**

$$\tau_{\max} = \frac{16T}{\pi d^3}$$

$$12(1000) = \frac{16T}{\pi(2^3)}$$

$$T = 18\,849.56 \text{ lb}\cdot\text{in}$$

$$T = \frac{P}{2\pi f}$$

$$18\,849.56 = \frac{P(396\,000)}{2\pi(240)}$$

$$P = 71.78 \text{ hp}$$

Problem 309 A steel propeller shaft is to transmit 4.5 MW at 3 Hz without exceeding a shearing stress of 50 MPa or twisting through more than 1° in a length of 26 diameters. Compute the proper diameter if $G = 83$ GPa.

$$T = \frac{P}{2\pi f} = \frac{4.5(1\,000\,000)}{2\pi(3)}$$

$$T = 238\,732.41 \text{ N}\cdot\text{m}$$

Based on maximum allowable shearing stress:

$$\tau_{\max} = \frac{16T}{\pi d^3}$$

$$50 = \frac{16(238\,732.41)(1000)}{\pi d^3}$$

$$d = 289.71 \text{ mm}$$

Based on maximum allowable angle of twist:

$$\theta = \frac{TL}{JG}$$

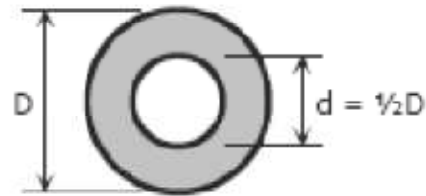
$$1^\circ \left(\frac{\pi}{180^\circ} \right) = \frac{238\,732.41(26d)(1000)}{\frac{1}{32} \pi d^4 (83\,000)}$$

$$d = 352.08 \text{ mm}$$

Use the bigger diameter, $d = 352 \text{ mm}$

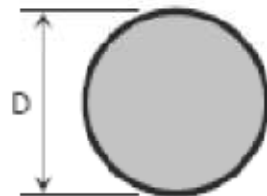
- **Problem 310** Show that the hollow circular shaft whose inner diameter is half the outer diameter has a torsional strength equal to 15/16 of that of a solid shaft of the same outside diameter.

Solution 310



Hollow circular shaft:

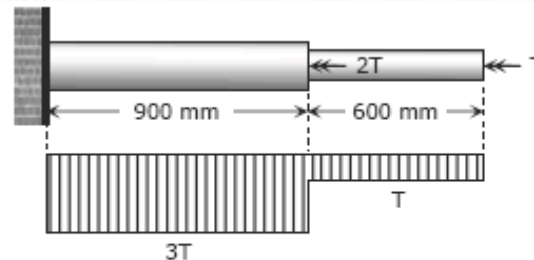
$$\begin{aligned}\tau_{\max\text{-hollow}} &= \frac{16TD}{\pi(D^4 - d^4)} \\ &= \frac{16TD}{\pi[D^4 - (\frac{1}{2}D)^4]} \\ &= \frac{16TD}{\pi(\frac{15}{16}D^4)} \\ &= \frac{16^2 T}{15\pi D^3}\end{aligned}$$



Solid circular shaft:

$$\begin{aligned}\tau_{\max\text{-solid}} &= \frac{16T}{\pi D^3} \\ &= \frac{15}{16} \left[\frac{16^2 T}{15\pi D^3} \right] \\ &= \frac{15}{16} \times \tau_{\max\text{-hollow}} \quad \text{ok!}\end{aligned}$$

- Problem 316** A compound shaft consisting of a steel segment and an aluminum segment is acted upon by two torques as shown in Fig. P-316. Determine the maximum permissible value of T subject to the following conditions: $\tau_{st} = 83$ MPa, $\tau_{al} = 55$ MPa, and the angle of rotation of the free end is limited to 6° . For steel, $G = 83$ GPa and for aluminum, $G = 28$ GPa.



Based on maximum shearing stress $\tau_{\max} = 16T / \pi d^3$:

$$\tau_{st} = \frac{16(3T)}{\pi(50^3)} = 83$$

$$T = 679\,042.16 \text{ N}\cdot\text{mm}$$

$$T = 679.04 \text{ N}\cdot\text{m}$$

$$\tau_{al} = \frac{16T}{\pi(40^3)} = 55$$

$$T = 691\,150.38 \text{ N}\cdot\text{mm}$$

$$T = 691.15 \text{ N}\cdot\text{m}$$

Based on maximum angle of twist:

$$\theta = \left(\frac{TL}{JG} \right)_{st} + \left(\frac{TL}{JG} \right)_{al}$$

$$6^\circ \left(\frac{\pi}{180^\circ} \right) = \frac{3T(900)}{\frac{1}{32} \pi (50^4) (83\,000)} + \frac{T(600)}{\frac{1}{32} \pi (40^4) (28\,000)}$$

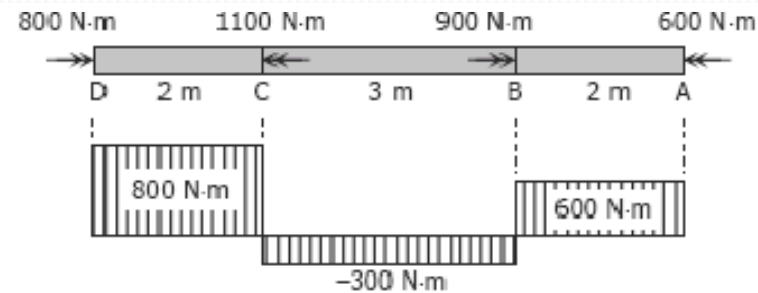
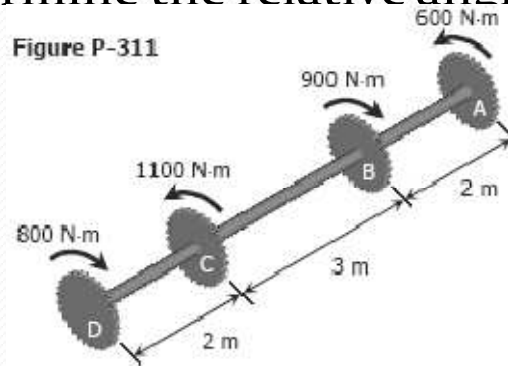
$$T = 757\,316.32 \text{ N}\cdot\text{mm}$$

$$T = 757.32 \text{ N}\cdot\text{m}$$

Use $T = 679.04 \text{ N}\cdot\text{m}$

Problem 311 An aluminum shaft with a constant diameter of 50 mm is loaded by torques applied to gears attached to it as shown in Fig. P-311. Using $G = 28 \text{ GPa}$, determine the relative angle of twist of gear D relative to gear A.

Figure P-311



$$\theta = \frac{TL}{JG}$$

Rotation of D relative to A:

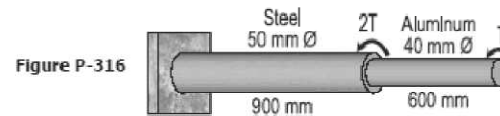
$$\theta_{D/A} = \frac{1}{JG} \sum TL$$

$$\theta_{D/A} = \frac{1}{\frac{1}{32} \pi (50^4) (28000)} [800(2) - 300(3) + 600(2)] (100^2)$$

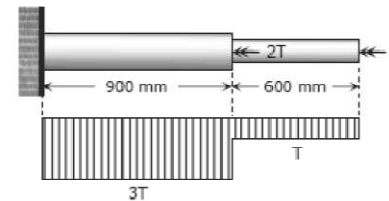
$$\theta_{D/A} = 0.1106 \text{ rad}$$

$$\theta_{D/A} = 6.34^\circ$$

- Problem 316** A compound shaft consisting of a steel segment and an aluminum segment is acted upon by two torques as shown in Fig. P-316. Determine the maximum permissible value of T subject to the following conditions: $\tau_{st} = 83 \text{ MPa}$, $\tau_{al} = 55 \text{ MPa}$, and the angle of rotation of the free end is limited to 6° . For steel, $G = 83 \text{ GPa}$ and for aluminum, $G = 28 \text{ GPa}$



Solution 316



Based on maximum shearing stress $\tau_{\max} = 16T / \pi d^3$:

$$\tau_{st} = \frac{16(3T)}{\pi(50^3)} = 83$$

$$T = 679\,042.16 \text{ N}\cdot\text{mm}$$

$$T = 679.04 \text{ N}\cdot\text{m}$$

$$\tau_{al} = \frac{16T}{\pi(40^3)} = 55$$

$$T = 691\,150.38 \text{ N}\cdot\text{mm}$$

$$T = 691.15 \text{ N}\cdot\text{m}$$

Based on maximum angle of twist:

$$\theta = \left(\frac{TL}{JG} \right)_{st} + \left(\frac{TL}{JG} \right)_{al}$$

$$6^\circ \left(\frac{\pi}{180^\circ} \right) = \frac{3T(900)}{\frac{1}{32}\pi(50^4)(83\,000)} + \frac{T(600)}{\frac{1}{32}\pi(40^4)(28\,000)}$$

$$T = 757\,316.32 \text{ N}\cdot\text{mm}$$

$$T = 757.32 \text{ N}\cdot\text{m}$$

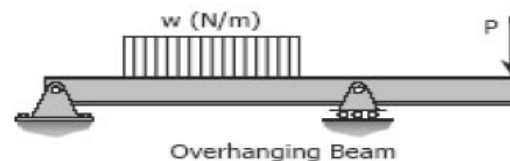
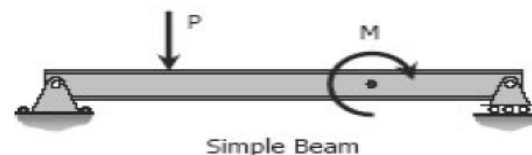
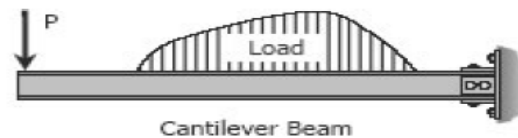
Use $T = 679.04 \text{ N}\cdot\text{m}$

- **Problem 317** A hollow bronze shaft of 3 in. outer diameter and 2 in. inner diameter is slipped over a solid steel shaft 2 in. in diameter and of the same length as the hollow shaft. The two shafts are then fastened rigidly together at their ends. For bronze, $G = 6 \times 10^6$ psi, and for steel, $G = 12 \times 10^6$ psi. What torque can be applied to the composite shaft without exceeding a shearing stress of 8000 psi in the bronze or 12 ksi in the steel?

- **Solve yourself.**

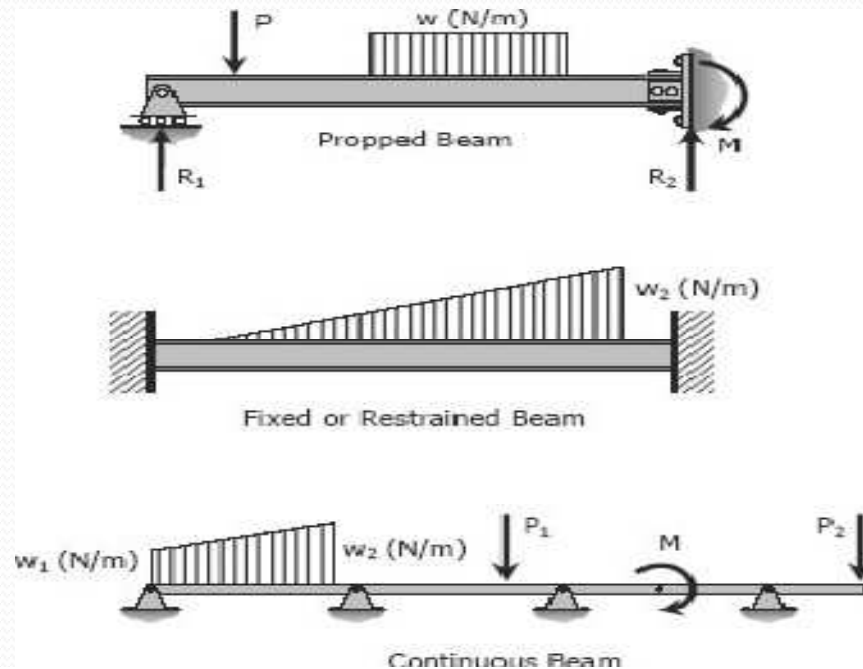
Shear & Moment in Beams

- DEFINITION OF A BEAM
- A beam is a bar subject to forces or couples that lie in a plane containing the longitudinal of the bar. According to determinacy, a beam may be determinate or indeterminate.
- **STATICALLY DETERMINATE BEAMS**
- Statically determinate beams are those beams in which the reactions of the supports may be determined by the use of the equations of static equilibrium. The beams shown below are examples of statically determinate beams.



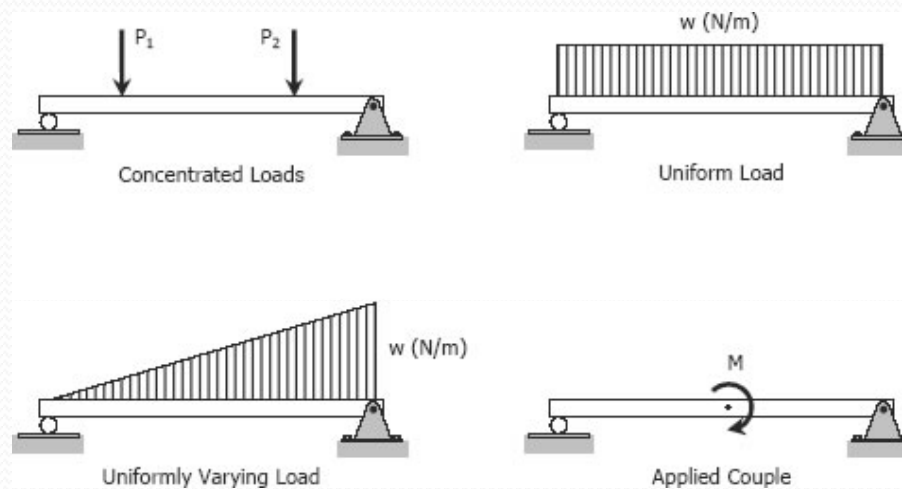
STATICALLY INDETERMINATE BEAMS

- If the number of reactions exerted upon a beam exceeds the number of equations in static equilibrium, the beam is said to be statically indeterminate. In order to solve the reactions of the beam, the static equations must be supplemented by equations based upon the elastic deformations of the beam.
- The degree of indeterminacy is taken as the difference between the number of reactions to the number of equations in static equilibrium that can be applied. In the case of the propped beam shown, there are three reactions R_1 , R_2 , and M and only two equations ($\sum M = 0$ and $\sum F_v = 0$) can be applied, thus the beam is indeterminate to the first degree ($3 - 2 = 1$).



TYPES OF LOADING

- Loads applied to the beam may consist of a concentrated load (load applied at a point), uniform load, uniformly varying load, or an applied couple or moment. These loads are shown in the following figures.



Relationship between Load, Shear, and Moment

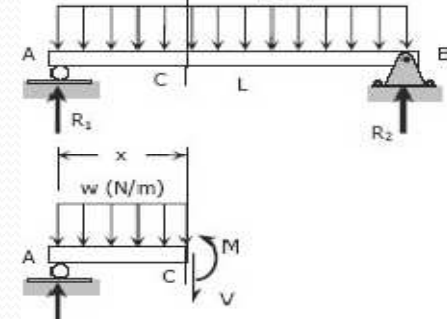
The vertical shear at C in the figure shown in previous section is taken as where $R_1 = R_2 = wL/2$

$$V_C = (\Sigma F_v)_L = R_1 - wx$$

$$V_C = \frac{wL}{2} - wx$$

$$M_C = (\Sigma M_C) = \frac{wL}{2}x - wx\left(\frac{x}{2}\right)$$

$$M_C = \frac{wLx}{2} - \frac{wx^2}{2}$$



If we differentiate M with

$$\frac{dM}{dx} = \frac{wL}{2} \frac{dx}{dx} - \frac{w}{2} 2x \frac{dx}{dx}$$

$$\frac{dM}{dx} = \frac{wL}{2} - wx = \text{shear}$$

Thus, the rate of change of the bending moment with respect to x is equal to the shearing force, or **the slope of the moment diagram at the given point is the shear at that point.** Thus

$$\frac{dM}{dx} = V$$

Thus, the rate of change of the bending moment with respect to x is equal to the shearing force, or **the slope of the moment diagram at the given point is the shear at that point.**

$$\frac{dV}{dx} = 0 - w = \text{load}$$

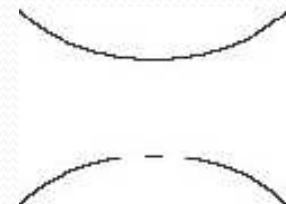
Differentiate V with respect to x gives

$$\frac{dV}{dx} = \text{Load}$$

Thus, the rate of change of the shearing force with respect to x is equal to the load or **the slope of the shear diagram at a given point equals the load at that point.**

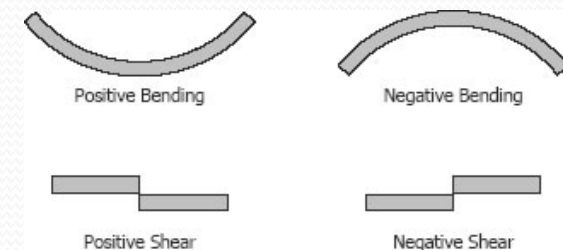
PROPERTIES OF SHEAR AND MOMENT DIAGRAMS

- The following are some important properties of shear and moment diagrams:
 - The area of the shear diagram to the left or to the right of the section is equal to the moment at that section.
 - The slope of the moment diagram at a given point is the shear at that point.
 - The slope of the shear diagram at a given point equals the load at that point.
- The maximum moment occurs at the point of zero shears. This is in reference to property number 2, that when the shear (also the slope of the moment diagram) is zero, the tangent drawn to the moment diagram is horizontal.
 - When the shear diagram is increasing, the moment diagram is concave upward.
 - When the shear diagram is decreasing, the moment diagram is concave downward.



SIGN CONVENTIONS

- The customary sign conventions for shearing force and bending moment are represented by the figures below.
- A force that tends to bend the beam downward is said to produce a positive bending moment. A force that tends to shear the left portion of the beam upward with respect to the right portion is said to produce a positive shearing force.
- An easier way of determining the sign of the bending moment at any section is that upward forces always cause positive bending moments regardless of whether they act to the left or to the right of the exploratory section.



Problem 425 Beam loaded as shown in Fig. P-425.

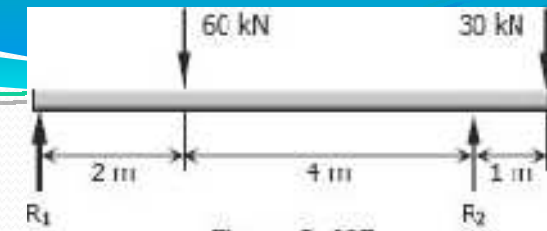
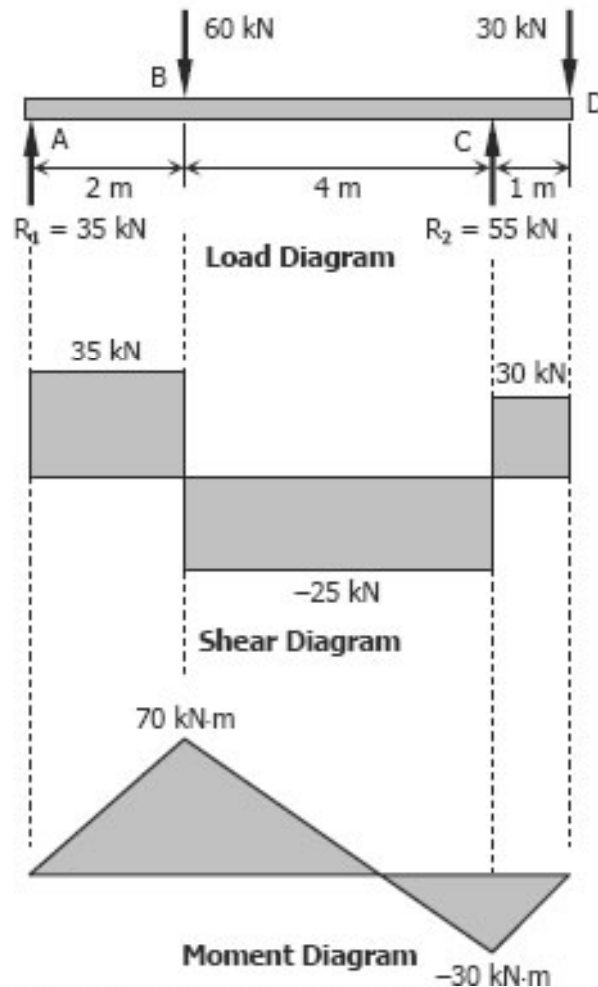


Figure P-425



$$\sum M_A = 0$$

$$6R_2 = 2(60) + 7(30)$$

$$R_2 = 55 \text{ kN}$$

$$\sum M_C = 0$$

$$6R_1 + 1(30) = 4(60)$$

$$R_1 = 35 \text{ kN}$$

To draw the Shear Diagram:

- (1) $V_A = R_1 = 35 \text{ kN}$
- (2) $V_B = V_A + \text{Area in load diagram} - 60 \text{ kN}$
 $V_B = 35 + 0 - 60 = -25 \text{ kN}$
- (3) $V_C = V_B + \text{area in load diagram} + R_2$
 $V_C = -25 + 0 + 55 = 30 \text{ kN}$
- (4) $V_D = V_C + \text{Area in load diagram} - 30 \text{ kN}$
 $V_D = 30 + 0 - 30 = 0$

To draw the Moment Diagram:

- (1) $M_A = 0$
- (2) $M_B = M_A + \text{Area in shear diagram}$
 $M_B = 0 + 35(2) = 70 \text{ kN}\cdot\text{m}$
- (3) $M_C = M_B + \text{Area in shear diagram}$
 $M_C = 70 - 25(4) = -30 \text{ kN}\cdot\text{m}$
- (4) $M_D = M_C + \text{Area in shear diagram}$
 $M_D = -30 + 30(1) = 0$

Problem 426 Cantilever beam acted upon by a uniformly distributed load and a couple as shown in Fig. P-426.

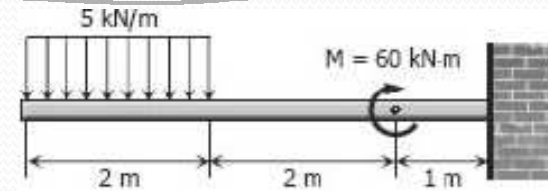
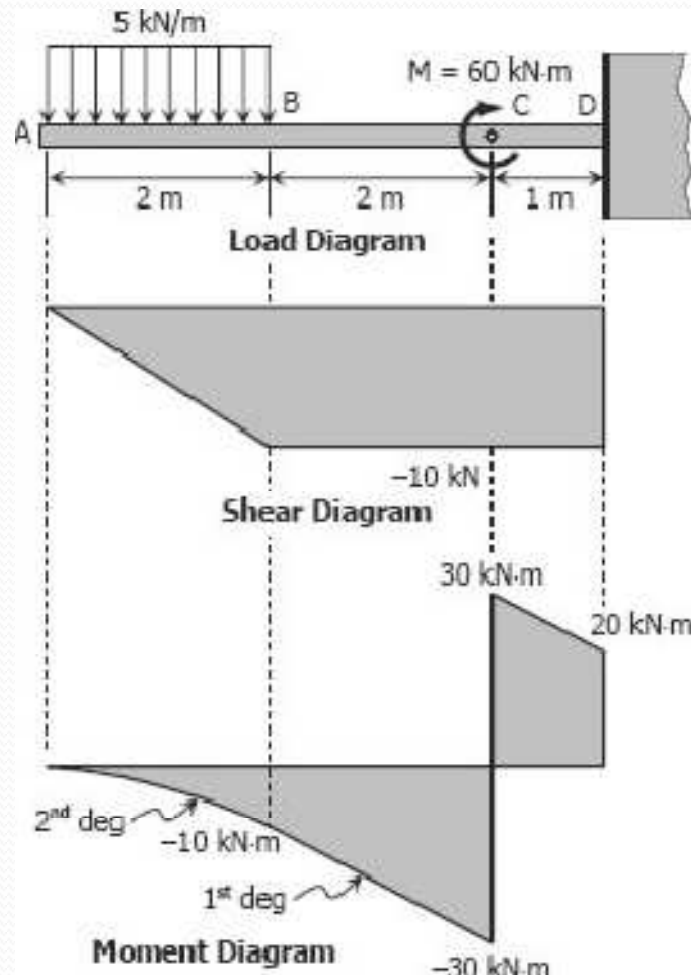


Figure P-426



To draw the Shear Diagram

- (1) $V_A = 0$
- (2) $V_B = V_A + \text{Area in load diagram}$
 $V_B = 0 - 5(2)$
 $V_B = -10 \text{ kN}$
- (3) $V_C = V_B + \text{Area in load diagram}$
 $V_C = -10 + 0$
 $V_C = -10 \text{ kN}$
- (4) $V_D = V_C + \text{Area in load diagram}$
 $V_D = -10 + 0$
 $V_D = -10 \text{ kN}$

To draw the Moment Diagram

- (1) $M_A = 0$
- (2) $M_B = M_A + \text{Area in shear diagram}$
 $M_B = 0 - \frac{1}{2}(2)(10)$
 $M_B = -10 \text{ kN}\cdot\text{m}$
- (3) $M_C = M_B + \text{Area in shear diagram}$
 $M_C = -10 - 10(2)$
 $M_C = -30 \text{ kN}\cdot\text{m}$
 $M_{CD} = -30 + M = -30 + 60 = 30 \text{ kN}\cdot\text{m}$
- (4) $M_D = M_C + \text{Area in shear diagram}$
 $M_D = 30 - 10(1)$
 $M_D = 20 \text{ kN}\cdot\text{m}$

Problem 428 Beam loaded as shown in Fig. P-428.

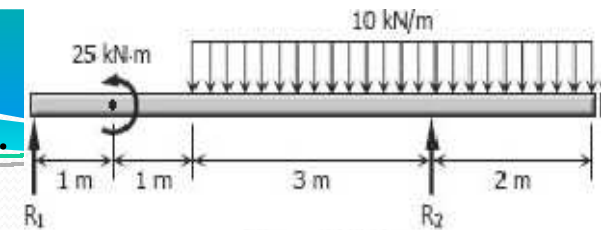
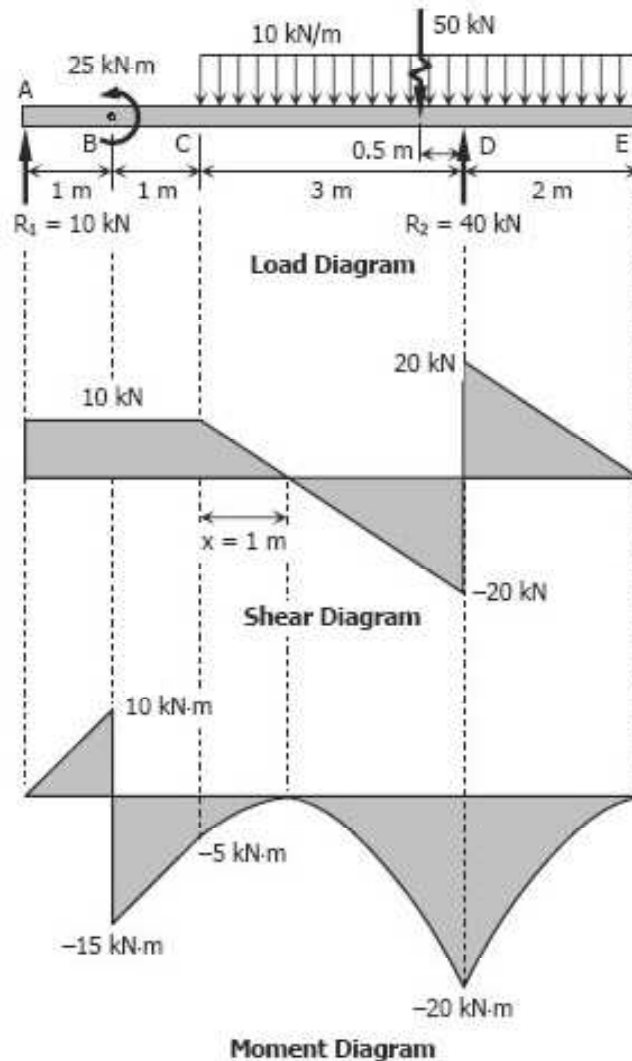


Figure P-428



$$\begin{aligned} \sum M_D &= 0 \\ 5R_1 &= 50(0.5) + 25 \\ R_1 &= 10 \text{ kN} \end{aligned}$$

$$\begin{aligned} \sum M_A &= 0 \\ 5R_2 + 25 &= 50(4.5) \\ R_2 &= 40 \text{ kN} \end{aligned}$$

To draw the Shear Diagram

- $V_A = R_1 = 10$ kN
- $V_B = V_A + \text{Area in load diagram}$
 $V_B = 10 + 0 = 10$ kN
- $V_C = V_B + \text{Area in load diagram}$
 $V_C = 10 + 0 = 10$ kN
- $V_D = V_C + \text{Area in load diagram}$
 $V_D = 10 - 10(3) = -20$ kN
 $V_{D2} = -20 + R_2 = 20$ kN
- $V_E = V_{D2} + \text{Area in load diagram}$
 $V_E = 20 - 10(2) = 0$
- Solving for x :
 $x / 10 = (3 - x) / 20$
 $20x = 30 - 10x$
 $x = 1$ m

To draw the Moment Diagram

- $M_A = 0$
- $M_B = M_A + \text{Area in shear diagram}$
 $M_B = 0 + 1(10) = 10$ kN-m
 $M_{B2} = 10 - 25 = -15$ kN-m
- $M_C = M_{B2} + \text{Area in shear diagram}$
 $M_C = -15 + 1(10) = -5$ kN-m
- $M_x = M_C + \text{Area in shear diagram}$
 $M_x = -5 + \frac{1}{2}(1)(10) = 0$
- $M_D = M_x + \text{Area in shear diagram}$
 $M_D = 0 - \frac{1}{2}(2)(20) = -20$ kN-m
- $M_E = M_D + \text{Area in shear diagram}$
 $M_E = -20 + \frac{1}{2}(2)(20) = 0$

• Problem 444 Beam loaded as shown in Fig. P-444

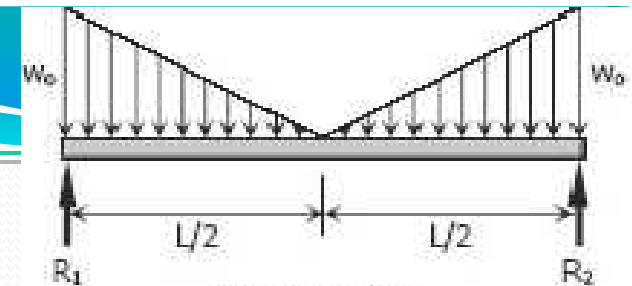
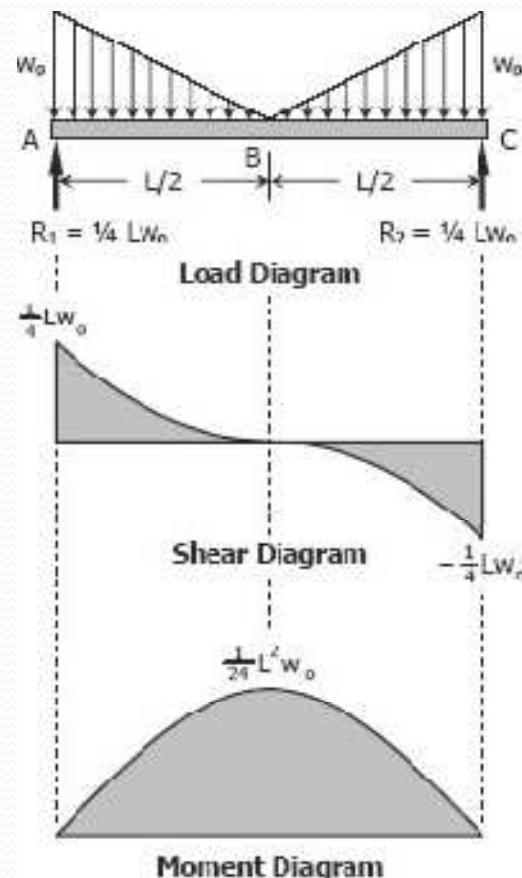


Figure P-444

$$\begin{aligned} \text{Total load} &= 2\left[\frac{1}{2}(L/2)(w_0)\right] \\ &= \frac{1}{2}Lw_0 \end{aligned}$$

By symmetry

$$\begin{aligned} R_1 = R_2 &= \frac{1}{2} \times \text{total load} \\ R_1 = R_2 &= \frac{1}{4}Lw_0 \end{aligned}$$



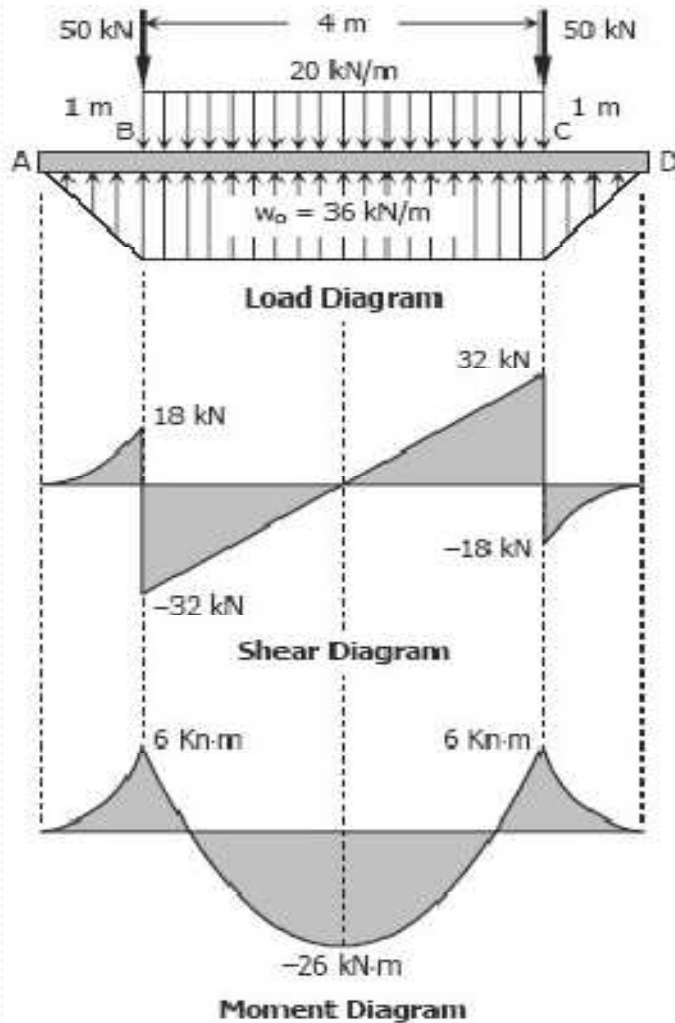
To draw the Shear Diagram

- (1) $V_A = R_1 = \frac{1}{4}Lw_0$
- (2) $V_B = V_A + \text{Area in load diagram}$
 $V_B = \frac{1}{4}Lw_0 - \frac{1}{2}(L/2)(w_0) = 0$
- (3) $V_C = V_B + \text{Area in load diagram}$
 $V_C = 0 - \frac{1}{2}(L/2)(w_0) = -\frac{1}{4}Lw_0$
- (4) The shear diagram in AB is second degree curve. The shear in AB is from $-w_0$ (downward w_0) to zero or increasing, thus, the slope of shear at AB is increasing (upward parabola).
- (5) The shear diagram in BC is second degree curve. The shear in BC is from zero to $-w_0$ (downward w_0) or decreasing, thus, the slope of shear at EC is decreasing (downward parabola)

To draw the Moment Diagram

- (1) $M_A = 0$
- (2) $M_B = M_A + \text{Area in shear diagram}$
 $M_B = 0 + \frac{1}{3}(L/2)(\frac{1}{4}Lw_0) = \frac{1}{24}L^2w_0$
- (3) $M_C = M_B + \text{Area in shear diagram}$
 $M_C = \frac{1}{24}L^2w_0 - \frac{1}{3}(L/2)(\frac{1}{4}Lw_0) = 0$
- (4) The shear diagram from A to C is decreasing, thus, the moment diagram is a concave downward third degree curve.

• **Problem 446** Beam loaded and supported as shown in Fig. P-446.



$$\sum F_v = 0$$

$$4w_o + 2\left[\frac{1}{2}w_o(1)\right] = 20(4) + 2(50)$$

$$5w_o = 180$$

$$w_o = 36 \text{ kN/m}$$

To draw the Shear Diagram

- (1) $V_A = 0$
- (2) $V_B = V_A + \text{Area in load diagram}$
 $V_B = 0 + \frac{1}{2}(36)(1) = 18 \text{ kN}$
 $V_{B2} = V_B - 50 = 18 - 50$
 $V_{B2} = -32 \text{ kN}$
- (3) The net uniformly distributed load in segment BC is $36 - 20 = 16 \text{ kN/m}$ upward.
 $V_C = V_{B2} + \text{Area in load diagram}$
 $V_C = -32 + 16(4) = 32 \text{ kN}$
 $V_{C2} = V_C - 50 = 32 - 50$
 $V_{C2} = -18 \text{ kN}$
- (4) $V_D = V_{C2} + \text{Area in load diagram}$
 $V_D = -18 + \frac{1}{2}(36)(1) = 0$
- (5) The shape of shear at AB and CD are parabolic spandrel with vertex at A and D, respectively.
- (6) The location of zero shear is obviously at the midspan or 2 m from B.

To draw the Moment Diagram

- (1) $M_A = 0$
- (2) $M_B = M_A + \text{Area in shear diagram}$
 $M_B = 0 + \frac{1}{3}(1)(18)$
 $M_B = 6 \text{ kN}\cdot\text{m}$
- (3) $M_{\text{midspan}} = M_B + \text{Area in shear diagram}$
 $M_{\text{midspan}} = 6 - \frac{1}{2}(32)(2)$
 $M_{\text{midspan}} = -26 \text{ kN}\cdot\text{m}$
- (4) $M_C = M_{\text{midspan}} + \text{Area in shear diagram}$
 $M_C = -26 + \frac{1}{2}(32)(2)$
 $M_C = 6 \text{ kN}\cdot\text{m}$
- (5) $M_D = M_C + \text{Area in shear diagram}$
 $M_D = 6 - \frac{1}{3}(1)(18) = 0$
- (6) The moment diagram at AB and CD are 3rd degree curve while at BC is 2nd degree curve.

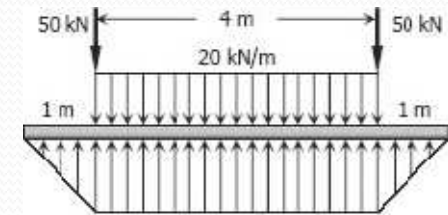
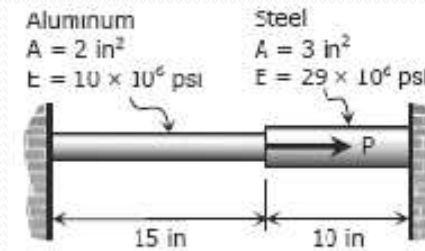


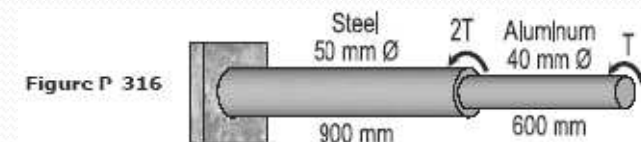
Figure P-446

Assignment

Problem 273 The composite bar shown in Fig. P-273 is firmly attached to unyielding supports. An axial force $P = 50$ kips is applied at 60°F . Compute the stress in each material at 120°F . Assume $\alpha = 6.5 \times 10^{-6}$ in/(in $\cdot^\circ\text{F}$) for steel and 12.8×10^{-6} in/(in $\cdot^\circ\text{F}$) for aluminum.



- Problem 316** A compound shaft consisting of a steel segment and an aluminum segment is acted upon by two torques as shown in Fig. P-316. Determine the maximum permissible value of T subject to the following conditions: $\tau_{st} = 83$ MPa, $\tau_{al} = 55$ MPa, and the angle of rotation of the free end is limited to 6° . For steel, $G = 83$ GPa and for aluminum, $G = 28$ GPa.



- Problem 417** For Beam carrying the triangular loading shown in Fig. P-417 draw the shear force and bending moment diagrams.

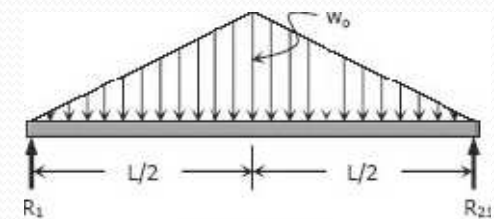


Figure P-417